



## GREEN INTERNATIONAL AFFILIATES, INC.

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January 11, 2020

Mr. Robert Gaynor  
Chairman, Zoning Board of Appeals  
Halifax Town Hall  
499 Plymouth Street  
Halifax, MA 02338

**Subject: Country Club Estates, Halifax, MA**  
**Response to Traffic Peer Review**

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Dear Mr. Gaynor,

On behalf of the applicant, Green International Affiliates, Inc. (Green) has reviewed the traffic related peer review comments<sup>1</sup> submitted by Gillon Associates dated May 28, 2020 and we have prepared this response to those comments. The Gillon review was done in regards to the traffic impact report<sup>2</sup> completed for the project.

In general, the peer review determined that the traffic impact study for the proposed development was done according to standard practices represented a reasonable assessment of impact of the proposed residential development of 30 units of housing. There was general concurrence in regard to the study area, based data and adjustments, site traffic forecasts and the key analysis findings related to site the access.

Mr. Gillon's primary concern revolves around the location of the site access point and the traffic signal operations, both at that point of new access as well as along the 3 signal system. As currently designed, the site drive will essentially be opposite the Stop & Shop (S&S) Drive but with a slight offset in relation to the S&S Drive centerline. Mr. Gillon indicated that signal timing could resolve any issue and requested that we examine a signal timing option that would "split" the signal phases for each drive. He also noted the final signal timing plans should be provided to the town for approval prior to construction and the need for the applicant to commit to design and construction of the modifications at the intersection.

In response, we completed additional signal timing analyses along Plymouth Street incorporating "split" phasing at the site drive intersection as well as testing various timing plans for the signal coordination. The analysis results showed that using split phasing at the site drive will more than adequately address the slight offset in alignment and maintain more than acceptable operating conditions. It will also be better given the large differential in volumes on the two opposite approaches.

Furthermore, the analysis showed that the split phasing along with optimized signal coordination timing would maintain acceptable levels of service at the 3 intersections as well as manage the peak hour vehicle queues as well as possible. The most recent signal analysis calculation sheets are attached.

In concluding, this assessment and review of the proposed 30 unit residential development at 314 Plymouth Street has indicated that the project is a low generator of traffic and will not have a significant impact on the traffic flow in the immediate are of the development. While this assessment has shown minimal anticipated vehicle traffic generation and the project can be safely accessed/egressed, several traffic safety and control related recommendations have been developed and the project can commit to the following building off the original recommendations contained in the October 2019 traffic report:

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<sup>1</sup> Gillon Associates, Traffic Study Peer Review Letter, Chapter 40B Development, 314 Plymouth Street, Halifax, MA, dated May 28, 2020.

<sup>2</sup> Green International Affiliates, Inc. Traffic Impact & Access Study, Proposed Halifax 40B Development, 314 Plymouth Street, Halifax, MA, October 2019.

- Upon approval of the project, the traffic signal at the Plymouth Street/Stop & Shop Drive will be modified to include a separate signal phase for the project drive. The applicant will be responsible for the design and construction of these modifications. All traffic control will conform to the MUTCD<sup>3</sup>.
- Traffic signal timing plans will be further refined during the design process and submitted for review by the town's peer traffic engineer consultant in order to reach agreement on the optimal signal timing.
- With the construction of the site drive, ADA compliant ramps will need to be constructed at the sidewalk locations and a crosswalk marked across the driveway to facilitate crossing Plymouth Street from the site to the Stop & Shop site.
- Any new vegetation or site signage adjacent to the site driveway on Pine Street should be kept low-lying (less than two feet tall) and/or set back sufficiently to maintain adequate sightlines.

If you have any questions, do not hesitate to contact me at 978-923-0400.

Very truly yours,  
**GREEN INTERNATIONAL AFFILIATES, INC.**

*William J Scully*

William J. Scully, P.E.  
Vice President

WJS/-

Attachments

Cc P. Cusson  
C. Tobias  
J. Gillon

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



















<sup>3</sup> U.S. Department of Transportation, Federal Highway Administration, Manual on Uniform Traffic Control Devices (MUTCD), Washington, D.C., 2009.

**Attachments**

# Synchro 10: Lanes, Volumes, Timings













## 2: Monponsett Street & Plymouth Street

Halifax 40B Development TIAS  
2026 Build Weekday PM Peak Hour Traffic Volumes

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	165	437	195	86	351	63	160	178	48	61	226	148
Future Volume (vph)	165	437	195	86	351	63	160	178	48	61	226	148
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	220		0	120		0	120		0	150		150
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	1736	1800	0	1787	1841	0	1770	1776	0	1770	1863	1599
Flt Permitted	0.358			0.127			0.950			0.950		
Satd. Flow (perm)	654	1800	0	239	1841	0	1770	1776	0	1770	1863	1599
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		33			13			15				172
Link Speed (mph)		25			25			25				25
Link Distance (ft)		405			1371			950				1117
Travel Time (s)		11.0			37.4			25.9				30.5
Peak Hour Factor	0.97	0.97	0.97	0.82	0.82	0.82	0.93	0.93	0.93	0.86	0.86	0.86
Heavy Vehicles (%)	4%	1%	0%	1%	1%	0%	2%	4%	2%	2%	2%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	170	652	0	105	505	0	172	243	0	71	263	172
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	1
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100		20	100		20	100		20	100	20
Trailing Detector (ft)	0	0		0	0		0	0		0	0	0
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	0
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		pm+pt	NA		Prot	NA		Prot	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8								6
Detector Phase	7	4		3	8		5	2		1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0

Synchro 10: Lanes, Volumes, Timings  
2: Monponsett Street & Plymouth Street

Halifax 40B Development TIAS  
2026 Build Weekday PM Peak Hour Traffic Volumes

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Split (s)	8.0	22.5		8.0	22.5		8.0	22.5		8.0	22.5	22.5
Total Split (s)	10.0	44.0		8.0	42.0		15.0	26.0		12.0	23.0	23.0
Total Split (%)	11.1%	48.9%		8.9%	46.7%		16.7%	28.9%		13.3%	25.6%	25.6%
Maximum Green (s)	7.0	41.0		5.0	39.0		12.0	23.0		9.0	20.0	20.0
Yellow Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	2.0
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
Lead/Lag	Lag	Lag		Lead	Lead		Lead	Lag		Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
Recall Mode	Min	C-Min		Min	C-Min		None	None		None	None	None
Walk Time (s)		7.0										
Flash Dont Walk (s)		12.0										
Pedestrian Calls (#/hr)		5										
Act Effect Green (s)	42.7	42.7		38.5	38.5		12.0	22.6		8.0	17.0	17.0
Actuated g/C Ratio	0.47	0.47		0.43	0.43		0.13	0.25		0.09	0.19	0.19
v/c Ratio	0.39	0.75		0.50	0.64		0.73	0.53		0.45	0.75	0.39
Control Delay	19.6	22.9		25.7	25.1		56.7	32.1		47.9	47.9	7.6
Queue Delay	0.0	1.1		0.0	0.1		0.0	0.0		0.0	0.0	0.0
Total Delay	19.6	24.0		25.7	25.2		56.7	32.1		47.9	47.9	7.7
LOS	B	C		C	C		E	C		D	D	A
Approach Delay		23.1			25.3			42.3			34.2	
Approach LOS		C			C			D			C	
Queue Length 50th (ft)	63	107		40	243		92	111		39	141	0
Queue Length 95th (ft)	92	212		61	276		#190	186		77	207	44
Internal Link Dist (ft)		325			1291			870			1037	
Turn Bay Length (ft)	220			120			120			150		150
Base Capacity (vph)	437	884		212	841		246	469		177	414	489
Starvation Cap Reductn	0	80		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	27		0	0		0	0	2
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.39	0.81		0.50	0.62		0.70	0.52		0.40	0.64	0.35

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green, Master Intersection

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.75

Intersection Signal Delay: 29.4

Intersection LOS: C

Intersection Capacity Utilization 73.7%

ICU Level of Service D

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

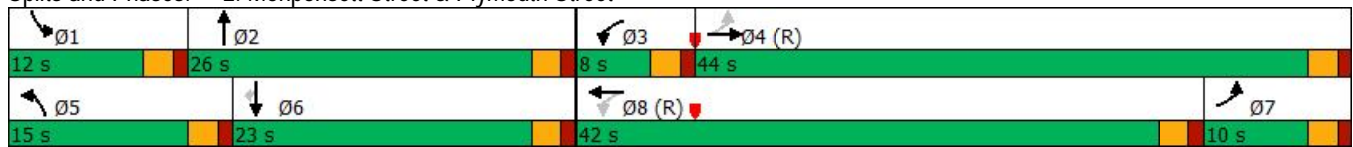
Queue shown is maximum after two cycles.

# Synchro 10: Lanes, Volumes, Timings

## 2: Monponsett Street & Plymouth Street

Halifax 40B Development TIAS  
2026 Build Weekday PM Peak Hour Traffic Volumes


















Splits and Phases: 2: Monponsett Street & Plymouth Street



# Synchro 10: Lanes, Volumes, Timings

# Halifax 40B Development TIAS













## 3: Site Driveway/Stop & Shop Driveway & Plymouth Street 2026 Build Weekday PM Peak Hour Traffic Volumes

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	13	653	8	12	685	70	5	0	7	94	0	25
Future Volume (vph)	13	653	8	12	685	70	5	0	7	94	0	25
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	225		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	1805	1896	0	0	1875	0	0	1676	0	0	1777	0
Flt Permitted	0.298				0.989						0.762	
Satd. Flow (perm)	566	1896	0	0	1856	0	0	1708	0	0	1407	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1			9			73			73	
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		533			578			431			399	
Travel Time (s)		14.5			15.8			11.8			10.9	
Peak Hour Factor	0.88	0.88	0.92	0.92	0.87	0.87	0.92	0.92	0.92	0.67	0.92	0.67
Heavy Vehicles (%)	0%	0%	2%	2%	0%	0%	2%	2%	2%	0%	2%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	15	751	0	0	880	0	0	13	0	0	177	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		7.0	7.0		7.0	7.0	


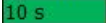

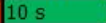






Lane Group	Ø9
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	9
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0



## 3: Site Driveway/Stop &amp; Shop Driveway &amp; Plymouth Street 2026 Build Weekday PM Peak Hour Traffic Volumes

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Split (s)	29.0	29.0		22.5	22.5		10.0	10.0		10.0	10.0	
Total Split (s)	52.0	52.0		52.0	52.0		10.0	10.0		10.0	10.0	
Total Split (%)	57.8%	57.8%		57.8%	57.8%		11.1%	11.1%		11.1%	11.1%	
Maximum Green (s)	49.0	49.0		49.0	49.0		7.0	7.0		7.0	7.0	
Yellow Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	
Total Lost Time (s)	3.0	3.0			3.0			3.0			3.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	C-Min	C-Min		C-Min	C-Min		None	None		None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	65.0	65.0			65.0			7.0				13.4
Actuated g/C Ratio	0.72	0.72			0.72			0.08				0.15
v/c Ratio	0.04	0.55			0.66			0.07				0.65
Control Delay	6.7	9.6			6.8			0.6				35.6
Queue Delay	0.0	0.0			0.0			0.0				0.0
Total Delay	6.7	9.6			6.8			0.6				35.6
LOS	A	A			A			A				D
Approach Delay		9.5			6.8			0.6				35.6
Approach LOS		A			A			A				D
Queue Length 50th (ft)	2	135			104			0				53
Queue Length 95th (ft)	13	427			#79			0				#195
Internal Link Dist (ft)		453			498			351				319
Turn Bay Length (ft)	225											
Base Capacity (vph)	408	1369			1343			200				271
Starvation Cap Reductn	0	0			0			0				0
Spillback Cap Reductn	0	0			0			0				0
Storage Cap Reductn	0	0			0			0				0
Reduced v/c Ratio	0.04	0.55			0.66			0.07				0.65
Intersection Summary												
Area Type:	Other											
Cycle Length: 90												
Actuated Cycle Length: 90												
Offset: 0 (0%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green												
Natural Cycle: 90												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 0.66												
Intersection Signal Delay: 10.7						Intersection LOS: B						
Intersection Capacity Utilization 68.6%						ICU Level of Service C						
Analysis Period (min) 15												
# 95th percentile volume exceeds capacity, queue may be longer.												
Queue shown is maximum after two cycles.												




















Splits and Phases: 3: Site Driveway/Stop & Shop Driveway & Plymouth Street

 Ø2  10 s	 Ø6  10 s	 Ø4 (R)  52 s	 Ø9  18 s
		 Ø8 (R)  52 s	

Lane Group	Ø9
Minimum Split (s)	18.0
Total Split (s)	18.0
Total Split (%)	20%
Maximum Green (s)	15.0
Yellow Time (s)	2.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	5.0
Flash Dont Walk (s)	10.0
Pedestrian Calls (#/hr)	5
Act Effect Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

Synchro 10: Lanes, Volumes, Timings  
6: Plymouth Street & WalMart Driveway

Halifax 40B Development TIAS  
2026 Build Weekday PM Peak Hour Traffic Volumes













												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	91	695	12	8	615	121	10	0	2	157	0	23
Future Volume (vph)	91	695	12	8	615	121	10	0	2	157	0	23
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	380		0	0		150	0		0	0		0
Storage Lanes	1		0	0		1	0		0	1		0
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	1787	1878	0	0	1880	1615	0	1782	0	1805	1615	0
Flt Permitted	0.214				0.991			0.854		0.742		
Satd. Flow (perm)	403	1878	0	0	1864	1615	0	1585	0	1410	1615	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1				92		73			542	
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		578			405			363			375	
Travel Time (s)		15.8			11.0			9.9			10.2	
Peak Hour Factor	0.89	0.89	0.89	0.87	0.87	0.87	0.50	0.50	0.50	0.79	0.79	0.79
Heavy Vehicles (%)	1%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	102	794	0	0	716	139	0	24	0	199	29	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100		20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0		0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0		0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	7	4			8			2			6	
Permitted Phases	4			8		8	2			6		
Detector Phase	7	4		8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	

Lane Group	Ø9
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	9
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s)	5.0

# Synchro 10: Lanes, Volumes, Timings

## 6: Plymouth Street & WalMart Driveway

Halifax 40B Development TIAS  
2026 Build Weekday PM Peak Hour Traffic Volumes

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Split (s)	8.0	22.5		28.0	28.0	28.0	8.0	8.0		8.0	8.0	
Total Split (s)	8.0	48.0		40.0	40.0	40.0	15.0	15.0		15.0	15.0	
Total Split (%)	8.9%	53.3%		44.4%	44.4%	44.4%	16.7%	16.7%		16.7%	16.7%	
Maximum Green (s)	5.0	45.0		37.0	37.0	37.0	12.0	12.0		12.0	12.0	
Yellow Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0			0.0	0.0		0.0		0.0	0.0	
Total Lost Time (s)	3.0	3.0			3.0	3.0		3.0		3.0	3.0	
Lead/Lag	Lead			Lag			Lag					
Lead-Lag Optimize?	Yes			Yes			Yes					
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	Min	C-Min		C-Min	C-Min	C-Min	None	None		None	None	
Walk Time (s)												
Flash Dont Walk (s)												
Pedestrian Calls (#/hr)												
Act Effct Green (s)	62.2	62.2			52.0	52.0		21.8		21.8	21.8	
Actuated g/C Ratio	0.69	0.69			0.58	0.58		0.24		0.24	0.24	
v/c Ratio	0.26	0.61			0.66	0.14		0.05		0.58	0.04	
Control Delay	7.1	9.2			13.5	3.1		0.2		37.7	0.1	
Queue Delay	0.0	0.1			1.0	0.0		0.0		0.0	0.0	
Total Delay	7.1	9.3			14.5	3.1		0.2		37.7	0.1	
LOS	A	A			B	A		A		D	A	
Approach Delay	9.0			12.7			0.3			32.9		
Approach LOS	A			B			A			C		
Queue Length 50th (ft)	14	122			158	4		0		101	0	
Queue Length 95th (ft)	m55	307			243	m15		0		142	0	
Internal Link Dist (ft)	498			325			283			295		
Turn Bay Length (ft)	380						150					
Base Capacity (vph)	389	1298			1077	972		438		341	801	
Starvation Cap Reductn	0	66			154	0		0		0	0	
Spillback Cap Reductn	0	21			35	0		1		0	10	
Storage Cap Reductn	0	0			0	0		0		0	0	
Reduced v/c Ratio	0.26	0.64			0.78	0.14		0.05		0.58	0.04	

### Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 89 (99%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.66

Intersection Signal Delay: 13.2

Intersection LOS: B

Intersection Capacity Utilization 89.8%

ICU Level of Service E

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Plymouth Street & WalMart Driveway



Synchro 10: Lanes, Volumes, Timings  
6: Plymouth Street & WalMart Driveway

Halifax 40B Development TIAS  
2026 Build Weekday PM Peak Hour Traffic Volumes

Lane Group	Ø9
Minimum Split (s)	27.0
Total Split (s)	27.0
Total Split (%)	30%
Maximum Green (s)	24.0
Yellow Time (s)	2.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	8.0
Flash Dont Walk (s)	16.0
Pedestrian Calls (#/hr)	0
Act Effect Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	